GOVERNMENT POLYTECHNIC VAISHALL Unit-6 Analysis and Design of T-Beam (LSM) Sub Code: 1615604 Sub: Design of stouetures (As per Syllabus) Civil Engg. By: Akash Kuman Lecturer Civi) General teatures of T-Beam: flang Plange Web. A T-Bears used in Construction, is a load bearing structure of reinforced Concrete, with a T-shaped Cross-section. The top of the T-shaped cross-section serves an a flange or comprension, member in resisting compressive sherres. The web (vertical section) of the beam below the compression flanges serves to resist shear shey.

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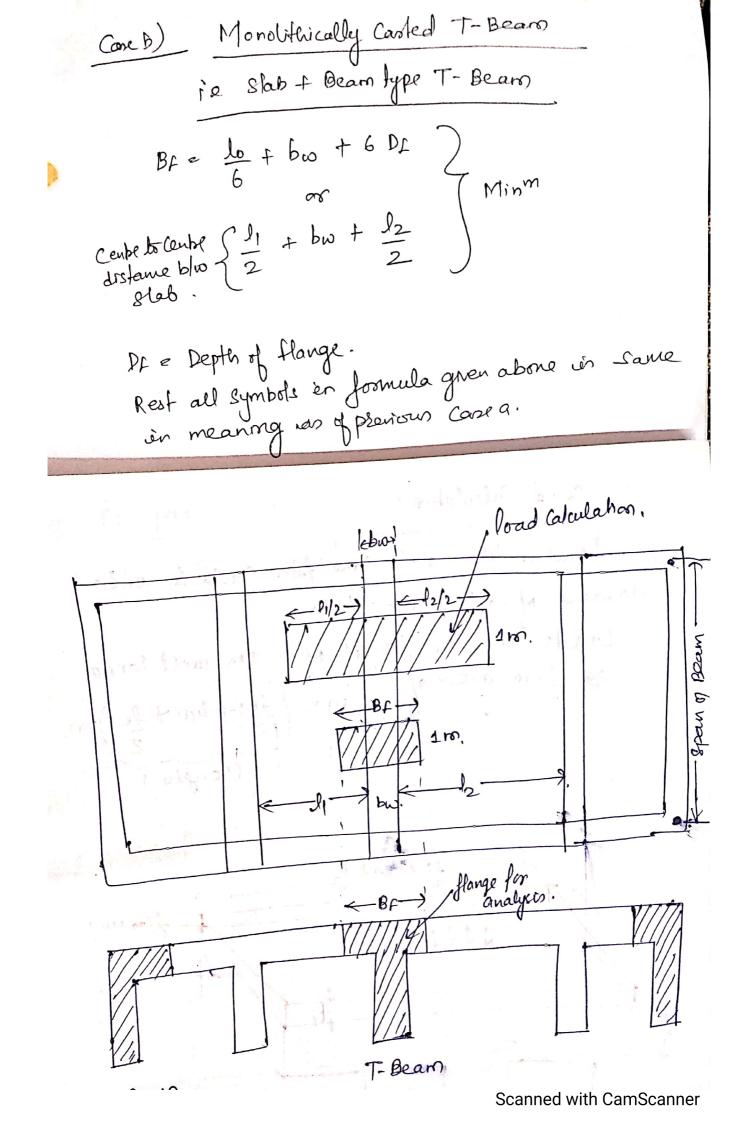


Advantages of T-Beams:

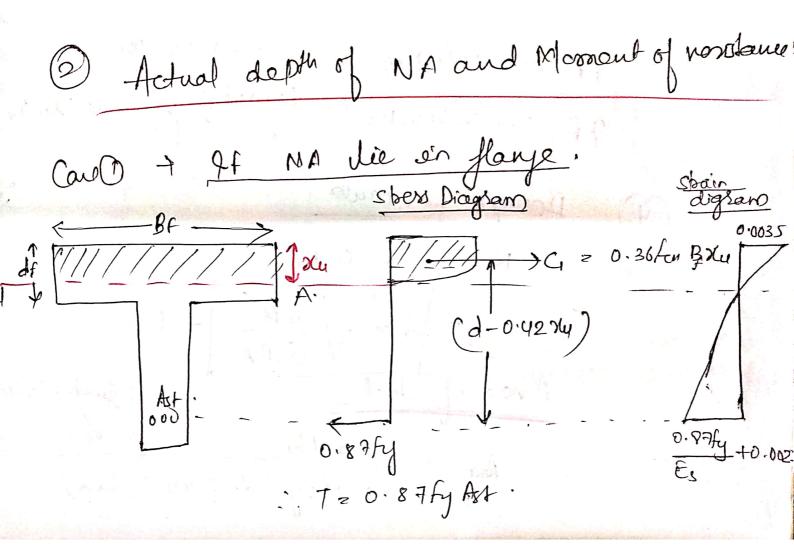
1> since the bearn in Cant monolithically with the slab, the flange also takes up the compressive spesses which mean, it will be more effective in resisting the sagging moment acting on the bean. 27 Better head room, this is direct outcome of the first point since the depth of beam can be considerably neduced 37 for larger spans, t-beams are usually proferred rather than rectangular beam as the deflection is reduced to a good extent. -> Effective width of Hange as por IS code: Actualsburgersteiburger deal shew for distribution Refer 15 458: 2000 Liagram (paratolic) b = actual width of flange. drapen pt = effective could hol reefangulon flange d = effective depth bf bu = width of webor reb or beam DF flonge of bears Df = depth of flange or slap. d web of Beam

Case a) Isofated T-Bears  $Bf = \frac{l_0}{\left(\frac{l_0}{B} + 4\right)} + bw$ where BF = effective width of flange bu = breadth of sibor web. B = actual width of flange. lo = distance b/w points of zero moment > 9F T Beam is simply Supported at ends. contration with an equi parmany. lo=leff. ( min + min BMD ~ lo=leff->9F T Beam is Continuo lfiped 89: -leff lo = 0.7 loff

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Analysis of singly - neinforced T- Bears As per syllabus we have to study only for the Case of neutral apris lying cost this the flange (1) Xu, lin ( Cimiting depth of neutral aprs). Xu, lin = Kd = (700) (1100+0.87fg))d e 0.53P -7 Fe 250 ~ 0.480 -> Fe 415 = 0.46 d -+ fe 500



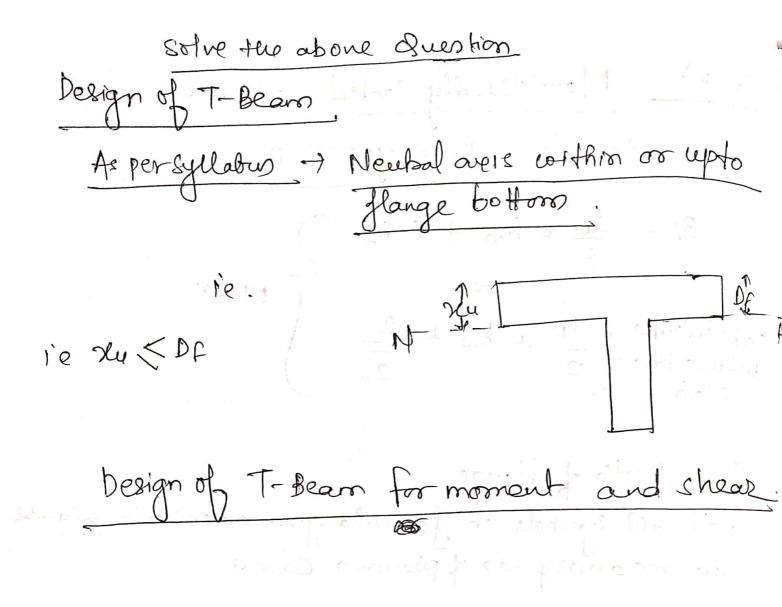
B.My 0.87 Fyjd. Z Ast 2 Case-2> 9F NiA us in web area. (Notin Syllabus) ie (du>Df)

Cabulate M.o. R of an isolated continous T-lean of effective span = 12m. UN M20, Le 415, LSM. 160 0mm 160 mm 840 mm 5-2500md 00000 60 rom leff=0-7/0 20.9×12 bw=400mm? 0-76 + 4 bio = 02400 + 400 0-76 + 4 bio = 02400 + 400 1600 + 4 bf = fa 2 08400 + 400 2 ± 443.47 mm 2-12-84-21-1000 2 130 8.108 mm Nu, 11m 2 0.48×840 = 403.200m Calculation Xth. Aminup Xa < DF.

a side a Ciperto, a bella 0.36 fen bf Xu 2 0.87 fy Az · du = 0.87 x 415x 5 x [[] / 21/2 0.36×30×1308.103 2 62.72 mm, JJF = 1600 mm < du, in (sours) Hence it is U.R.S and assemptions also correct . Nu 2 0.36 For BF 24 ( d- 0.42 Xy) 2 0-36 × 30× 1308.108×62.72 840-0.42×62.72 = 720.97 KN-M



- If depth of slab is 10 cm, width of web 30 cm, depth of web 50 cm, centre to centre distance of beams 3 m, effective span of beams 6 m, the effective flange width of the beam, is
  - [A]. 200 cm
  - [B]. 300 cm
  - [C]. 150 cm
  - [D]. 100 cm



It Coad Calculation . Load on bean well come poon centre le centre defance of adjacent slabs. wads over in layty of bean will come 62 farm area of  $1 m \left(\frac{p}{2} + b\omega + \frac{p}{2}\right) m$ width (Leugth ) (W  $\frac{1}{2}$  + bw +  $\frac{1}{2}$ Joolen JJJJ Line Load (D-8F) bearn Im Eboo-Mr XIWX ( 1 + but 10) = Live load 2  $(\mathbf{f})$ z tf XIm x ( 1 + bw + ln) x wh z g formy) W2 floorn (2)z de XIMX (le + bootle) x We z (United weight of stab 6 ح Wz of conorele) veright 2 bw xImx (D-df) XWc web prohond bean Scanned with CamScanner

- Designing for moment. 9F BMU, < MRU, Su, < dF. There first care. a) Find our sty, by equating BMU, = MB 1è B.M. UI 2 0.36 feu Br Dui (d-0.42 dui) 80 get 241 = 9

b) Area of steel:  
B. 
$$M_{u_1} \ge 0.87 \text{ fy} \text{ Asr} (d-0.42 \text{ du})$$
  
 $\therefore \text{ Asr} \ge \frac{B M u_1}{0.87 \text{ fy} (d-0.42 \text{ du})} \xrightarrow{(P)}$ 

Or A Ibeam as shown in fig in simply supposed onen an effectue span of 1000. Design the bearn; if the bears in Subjected to following B.M's. Use M25, fogoo, factored Moment Design For (D) 450 KN-M Z= 675 reno-m

$$\frac{2000 \text{ torm}}{1000 \text{ torm}}$$

$$\frac{900 \text{ torm}}{1000 \text{ torm}}$$

$$\frac{900 \text{ torm}}{1000 \text{ torm}}$$

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$$\frac{800 \text{ finding 19.0 \text{ R} at specyfed locabran.}}{1000 \text{ torm}}$$

$$\frac{800 \text{ MRu}_{1} = 0.36 \text{ few bx dx} (d - 0.42 \text{ dx})$$

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$$\frac{10,000}{2000} \text{ ty}$$

CAR AND AND (ave 1) when BMU, = 1.5×450 = 675 kp-m. 10 BMU, < MRU, of SourdE? Ist Cono. Equating : B·MU1 = 0.36 feu. Bf. Nu, (d-0.4×u,) 675×10 ° z 0.36 × 25 × 1471.11 ×4, (900-0,42×4,) (11) du, z 58.23 mm (125 mm REASHIE COPPARINE So Ast 2 0.36 feu Bf Xu, 0.87 Fy. ar Sant at 2 0.36 p25 × 1471.11 × 58.23 101 (151 0: 0· 87 × 500 1772-3300m2. So provide 5-20mm & bars.

· Design of T-Bears for shear will be same as studied as Unif-5 of Syllabus. Types of problems 1) finding effective flange width. 2) finding moment of vesistance of T-Bear Section with N.A files within or upto the bottoon of flange ie (xu < df) ( shall be the and a lot